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Traffic Engineering Assessment

Proposed Mixed-Use Development 1041 Centre Road, Oakleigh (Links Shopping Centre)

Prepared for 1041 Centre Road Pty Ltd

April, 2022

G29458R-01C

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1. Introduction

Traffix Group has been engaged by 1041 Centre Road Pty Ltd to prepare a traffic engineering assessment for a proposed mixed-use development at 1041 Centre Road, Oakleigh (Links Shopping Centre).

This report provides our traffic engineering assessment of the parking and traffic issues associated with the proposed development.

2. Existing Conditions

2.1. Site Locality

The development site is located on the north side of Centre Road, approximately 200m east of Warrigal Road in Oakleigh, and is part of the larger subject site known as the Links Shopping Centre, as presented in the locality plan at Figure 1 below.



Figure 1: Locality Map

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The development site is occupied by a warehouse building comprising approximately 3,300m² of floor area and is used by Harvey Norman as a product pickup/storage location.

It has an area of approximately 5,000m² and frontages to Centre Road and Links Avenue (two frontages) of approximately 47.5m, 109m(north-south) and 50m (east-west) respectively. Vehicle access is taken from the north-south section of Links Avenue at the northern boundary to the site. The subject site is provided access via two signalised access points (one via Centre Road and one via Warrigal Road).



An aerial view of the site is shown in Figure 2 below.

Figure 2: Aerial Photograph (April 2021)



2.2. Land Use

The site is zoned Commercial Zone – Schedule 1 (C1Z) as shown in Figure 3 below.

It is affected by a Design and Development Overlay - Schedule 1 (DDO1).





The development site is located within the Links Shopping Centre which provides a range of commercial uses including Woolworths Supermarket and Bunnings Warehouse. Land beyond the commercial uses fronting Centre Road and Warrigal Road is generally zoned as residential.

2.3. Road Network

Centre Road is an arterial road under the control of the Department of Transport (DoT) and is within a Road Zone Category 1 (RDZ1).

It extends approximately 13.2km in an east-west direction between Springvale Road to the east (where it continues as Police Road) and Hampton Street to the west.

In the vicinity of the development site, Centre Road is constructed with a 12.5m wide carriageway comprising two through traffic lanes in each direction.

Kerbside parking is not permitted on either side of the road.

A posted speed limit of 60km/h applies.





Figure 4: Centre Road view West

Figure 5: Centre Road view East

Warrigal Road is state arterial road located within a Road Zone Category 1 (RDZ1) and under the control of the Department of Transport (DoT). It extends approximately 19km in a north-south direction between Canterbury Road to the north and Beach Road to the south.

In the vicinity of the development site, Warrigal Road is constructed as an undivided carriageway with two lanes in each direction with flaring and merging at the major intersection with Centre Road.

Parking is prohibited along Warrigal Road.

A posted speed limit of 60km/h applies.

Links Avenue is private road within the Links Shopping Centre precinct. It extends east from Warrigal Road for approximately 240m and then turns south to Centre Road (approximately 125m).

Both of its intersections with the arterial roads are controlled by signals.

Links Avenue is generally constructed with a 7.3m wide carriageway which flares at the signalised intersections. A footpath is provided along the south and west sides.

Kerbside parking is not permitted along Links Avenue.

A posted speed limit of 20km/h applies.



Traffic Engineering Assessment

1041 Centre Road, Oakleigh (Links Shopping Centre)



Figure 6: Links Avenue view North



Figure 7: Links Avenue view South



Figure 8: Links Avenue view West



Figure 9: Links Avenue view East



2.4. **Principal Public Transport Network**

The subject site is located within the Principal Public Transport Network area as shown in Figure 10 below.





Principal Public Transport Network Area

Figure 10: Excerpt of Principal Public Transport Network Area (Monash Council)



2.5. Public Transport

The development site is well located with regard to public transport. The following public transport services operate within 500m of the subject site.

- Bus route 703 operates along Centre Road directly past the subject site. It provides a connection between Middle Brighton and Blackburn via Bentleigh, Clayton and Monash University.
- Bus route 733 operates along Centre Road and Cameron Avenue with the nearest stop approximately 400m east of the site on Centre Road. It provides a connection between Oakleigh and Box Hill via Clayton, Monash University and Mt Waverley.
- Bus route 903 is a SMARTBUS route which operates along Warrigal Road directly past the subject site. It provides a connection between Altona and Mordialloc via Sunshine, Heidelberg, Burwood, and Mentone.

The above services also provide connection to other public transport routes and services providing greater connectivity to the greater metropolitan area.



Figure 11 shows the nearby public transport services.

Figure 11: Excerpt of Monash Public Transport Map

2.6. Existing Traffic Conditions

Traffix Group has sourced SCATS data for the intersection of Centre Road/Warrigal Road from DoT's open data portal. The data is from Thursday 6th February 2020, prior to any traffic impacts as a result of COVID 19.

A summary of the turning movement data is presented at Figure 12 below.



Figure 12: Existing Turning Movement Counts – AM (PM)

3. Proposal

The proposal is for multi-storey mixed-use buildings comprising the uses specified in Table 1 below.

Table 1: Schedule of Uses

Use		No./Measure		
Dwellings	Studio	7 units		
	One-Bedroom	112 units		
	Two-bedroom	54 units		
	TOTAL	173 units		
Supermarket		1,913m ²		
Retail		875m ²		
Car Parking		238 spaces (178 basement and 60 at-grade on the east side of Links Avenue)		
Bicycle Parking		212 spaces		

Car parking for the residential use will be provided within a basement accessed via a new crossover in the northwest corner of the site via Links Avenue. The at-grade parking will be provided on the west side of Links Avenue. A pedestrian crossing will be provided to facilitate safe movements between the at-grade parking and the proposed uses.

A designated loading area is provided along the west side of the development with access to the loading area provided via a single width industrial crossover suitable for vehicles up to 19m semi-trailers.

The proposal includes minor amendments to the existing Links Shopping Centre carpark to facilitate suitable access to the loading area.

A copy of the proposed development plans prepared by i2C Architects (dated February 2022) is attached at Appendix A.



4. Car Parking Assessment

4.1. Statutory Car Parking Requirement

Clause 52.06 of the Planning Scheme sets out the statutory requirements for car parking. The purposes of Clause 52.06 are:

- To ensure that car parking is provided in accordance with the Municipal Planning Strategy and the Planning Policy Framework.
- To ensure the provision of an appropriate number of car parking spaces having regard to the demand likely to be generated, the activities on the land and the nature of the locality.
- To support sustainable transport alternatives to the motor car.
- To promote the efficient use of car parking spaces through the consolidation of car parking facilities.
- To ensure that car parking does not adversely affect the amenity of the locality.
- To ensure that the design and location of car parking is of a high standard, creates a safe environment for users and enables easy and efficient use.

The statutory car parking requirement is set out in Table 2 below.

Use	Measure	Rate	Requirement ⁽¹⁾
Dwelling	173 one- or two- bedroom units	1 space to each one or two bedroom unit	173 spaces
Supermarket	1,913m ²	5 spaces to each 100m ² of leasable floor area	95 spaces
Shop	875m ²	3.5 spaces to each 100m ² of leasable floor area	30 spaces
TOTAL			298 spaces

Table 2: Statutory Car Parking Requirement

(1) Rounded down in accordance with the Planning Scheme.

The proposed development has a statutory requirement for 298 car parking spaces.

The proposal includes provision of 178 spaces within the basement to satisfy the residential component. A further 60 at-grade parking spaces are provided on the east side of Links Avenue for the commercial uses. The provision of commercial spaces falls short of the



statutory car parking requirement and accordingly, a permit to reduce the car parking requirement is being sought as part of this application.

4.2. Reducing the Car Parking Requirement

Practice Note 22 (June 2012) specifies that the provisions draw a distinction between the assessment of likely demand for parking spaces, and whether it is appropriate to allow the supply of fewer spaces. These are two separate considerations, one technical while the other is more strategic. Different factors are taken into account in each consideration.

Accordingly, the applicant must satisfy the responsible authority that the provision of car parking is appropriate on the basis of a two-step process, which has regard to:

- The car parking demand likely to be generated by the use.
- Whether it is appropriate to allow fewer spaces to be provided.

An assessment of the appropriateness of reducing the car parking provision below the statutory requirement is set out below.

4.3. Car Parking Demand

Residential

For this development, the statutory car parking requirement for the residential component is expected to be reflective of the expected car parking demand.

Supermarket

In shopping centres, where there is a major full-line supermarket, the inclusion of a smaller secondary supermarket serves to supplement shoppers with further choices and does not generate car parking at the full statutory rate.

In our experience, supermarkets of a similar size typically generate parking at a rate of 3.5 spaces per 100m² of leasable floor area. Accordingly, the 2,000m² supermarket is expected to generate a parking demand for 67 spaces.

<u>Shop</u>

We have adopted the statutory rate as reflective of the retail/shop car parking demands for this development, i.e. 30 spaces.

Summary

The expected car parking demand for the proposed development is summarised in Table 3 below.



Table 3: Expected Car Parking Demand

Use	Parking Demand
Dwelling	173 spaces
Supermarket	67 spaces
Shop	30 spaces
TOTAL	270 spaces

4.4. Providing Fewer Car Parking Spaces

The second step in determining whether it is appropriate to reduce the statutory car parking requirement is to consider whether it is appropriate to allow fewer spaces to be provided than the number likely to be generated by the site. Traffix Group has undertaken assessments based on the relevant decision guidelines specified at Clause 52.06-7 of the Planning Scheme below.

Car Parking Demand Assessment

The proposed development is expected to generate a maximum car parking demand for 270 spaces including 97 commercial spaces. The residential component of the parking demand is met on-site however, the provision of 60 at-grade commercial spaces results is a shortfall of 37 spaces.

Availability of Alternative Car Parking

The existing Links Shopping Centre has a surplus of approximately 52 car parking spaces which are not leased as part of the existing buildings. The surplus of 52 spaces is more than sufficient to meet the proposed car parking shortfall associated with the commercial component of the development.

Future Growth and Development of Activity Centre

The proposed development represents the growth and development of the Links Shopping Centre activity centre. The former approvals included surplus car parking which would be made available to facilitate the growth of the centre. Accordingly, there will be no adverse impacts to the viability of the overall shopping precinct as a result of the proposed car parking shortfall.

Conclusion

Having regard to the decision guidelines above, we believe that there is sufficient justification to warrant a reduction in the statutory car parking requirement for this development.



4.5. Car Park Layout and Access

The proposed car parking layout and access arrangements have been assessed under the relevant sections of the Planning Scheme and the relevant Australian Standards.

Key elements of the design include:

- All vehicles can enter and exit the site in a forwards direction.
- All accessways are designed to accommodate simultaneous two-way traffic.
- A minimum headroom clearance of 2.1m is provided throughout the basement.
- All 90-degree parking spaces are provided in accordance with the minimum dimensions set out at Clause 52.06-9 Design Standard 2.
- Columns are located in accordance with the Clearance Diagram at Clause 52.06-9 Design Standard 2.
- A turnaround bay is provided at the end of the blind-aisle for the commercial carparking area.
- A DDA space is provided within the new at-grade car park for the retail uses in accordance with AS/NZS 2890.6:2009.
- A maximum ramp grade of 1:10 is provided for the first 5 metres from the property boundary.
- A maximum ramp grade of 1:4.1 is provided on the site (residential parking).
- Appropriate ramp transitions are provided to prevent scrapping and bottoming out of vehicles.

We are satisfied that the proposed car parking layout meets the relevant requirements of the Planning Scheme and Australian Standard and importantly, will work well.



5. Traffic Assessment

5.1. Traffic Generation

Residential

The RTA Guide to Traffic Generating Developments (2002) (RTA Guide) sets out traffic generation rates based on survey data collected in New South Wales for a range of land uses. This guide is referred to in the Austroads Guide which is used by VicRoads, and is generally regarded as the standard for metropolitan development characteristics.

The RTA Guide sets out the following relevant traffic generation rates for medium density residential development:

Smaller Units (one and two bedrooms):

- Daily vehicle trips = 4 5 per dwelling per day
- Weekday peak hour vehicle trips = 0.4 0.5 per dwelling per day

Having regard to the locality of the site, the congested nature of the surrounding arterial roads, the availability of alternative transport modes and the limited supply of car parking at nearby railways stations, we believe that an appropriate peak hour traffic generation rate is 0.3 per dwelling. Accordingly, the proposed development (173 dwellings) will generate in the order of 52 vehicle trip ends during each peak hour. This corresponds to one vehicle movement every 70 seconds on average.

Commercial

The commercial component of the new development will not generate traffic at the same rate as a single standalone full line supermarket and will result in a longer duration of stay on average as customers look to take advantage of the second supermarket and additional retail space.

The relationship between duration of stay, car parking and traffic generation are all interlinked with traffic generation inversely proportional to duration of stay.

Based on the above, we expect that the traffic generated by the new commercial component will be approximately 50% of the RTA rate for supermarkets/shopping centres.

Accordingly, the commercial component will generate a PM peak of 171 movements.

In our experience, significantly less traffic is generated to a shopping centre during the AM peak period, i.e. in the order of 35% of the PM peak. There will be 60 vehicle movements during the AM peak.



5.2. Traffic Distribution

Residential

The expected traffic distribution for the residential component of the development is as follows:

<u>AM Peak</u>

- 10% IN and 90% OUT,
- 50% to/from the north,
- 23% to/from the south,
- 7% to/from the west, and
- 20% to/from the east.

<u>PM Peak</u>

- 75% IN and 25% OUT,
- 50% to/from the north,
- 23% to/from the south,
- 7% to/from the west, and
- 20% to/from the east.

Commercial

The commercial component of the traffic generation is expected to follow the same distribution pattern as the existing use and is set out below.

<u>AM Peak</u>

- 50% IN and 50% OUT,
- 34% to/from the north,
- 23% to/from the south,
- 11% to/from the west, and
- 32% to/from the east.

<u>PM Peak</u>

- 50% IN and 50% OUT,
- 33% to/from the north,
- 20% to/from the south,
- 13% to/from the west, and
- 34% to/from the east.

A turning movement diagram is presented at Figure 13 which shows the expected site generated traffic movements.



Figure 13: Site Generated Traffic Distribution Diagram

5.3. Traffic Impact

Traffix Group has prepared a SIDRA intersection analysis for the signalised intersections providing access to the Links Shopping Centre and the intersection of Centre Road/Warrigal Road in both the AM and PM peak periods. These models have been linked in the network function due to their close proximity to one another and to accurately reflect the operating conditions of the road network.

It is significant to note that the existing traffic data was collected in February 2020, prior to any traffic impacts from COVID19. Due to the significant reduction in traffic as a result of COVID 19 and the change in travel to work behaviour, it was not possible to calibrate the model based on our recent observations to the pre-COVID conditions.

Accordingly, we have made the following assumptions in order to show an existing traffic analysis with a degree of saturation of approximately 1.0:

- · reduction in the inter-green time (yellow time reduced to 3 seconds),
- area type factor increased to 1.1,
- · signal coordination between the intersections,
- · favourable arrival type for external approaches to the network,
- due to the highly congested nature of the road network, traffic from the north into the Warrigal Road access will be passer-by trips only,
- all other traffic is assumed to be new traffic on the road network, and
- through lane capacity increased for Warrigal Road and Centre Road to 2,200 vehicles per hour.

In inner urban areas, where traffic is highly congested, it is not unusual for a degree of saturation to be around 1.0 which indicates that the intersection is at capacity.

For the purposes of a robust assessment, all site generated traffic is considered to be new traffic on the road network for this analysis.

A summary of the key outputs from the SIDRA model are presented as follows:

- Table 4 existing and post development site access via Warrigal Road,
- Table 5 existing and post development site access via Centre Road, and
- Table 6 existing and post development Centre Road/Warrigal Road intersection.

le de la constante de la const	Degree of Saturation		Average Delay (sec)		95 th Percentile queue (m)		
Approach	Existing	Post Development	Existing	Post Development	Existing	Post Development	
AM Peak Hour							
South Warrigal Road	0.474	0.474	0.7 sec	0.8 sec	20.3m	20.2m	
East Links Avenue	0.433	0.641	41.1 sec	45.7 sec	18.8m	28.5m	
North Warrigal Road	0.892	0.901	11.8 sec	13.7 sec	115.0m	128.2m	
PM Peak Hour							
South Warrigal Road	0.480	0.483	0.9 sec	1.1 sec	24.2m	27.0m	
East Links Avenue	0.973	0.932	61.6 sec	55.7 sec	51.1m	61.4m	
North Warrigal Road	0.881	0.890	9.8 sec	11.6 sec	284.5m	281.8m	

Table 4: Summary of SIDRA Outputs (Site Access via Warrigal Road)

The site access via Warrigal Road operates at a high degree of saturation as a result of the significant through volumes on Warrigal Road and the queues that extend from the intersection of Centre Road/Warrigal Road.

The analysis shows that there is no material change to any of the key operational outputs during the commuter peak periods as a result of the proposed development.



	Degree of Saturation		Average Delay (sec)		95 th Percentile queue (m)	
Approach	Existing	Post Development	Existing	Post Development	Existing	Post Development
AM Peak Hour						
East Centre Road	0.906	0.911	10.4 sec	11.7 sec	50.8m	58.6m
North Links Avenue	0.143	0.174	12.5 sec	13.6 sec	10.1m	13.5m
West Centre Road	0.310	0.312	15.6 sec	16.9 sec	146.8m	151.4m
PM Peak Hour						
East Centre Road	0.358	0.895	10.9 sec	19.7 sec	57.1m	103.5m
North Links Avenue	0.216	0.275	16.1 sec	16.5 sec	22.8m	31.9m
West Centre Road	0.362	0.359	30.0 sec	27.3 sec	150.0m	152.5m

Table 5: Summary of SIDRA Outputs (Site Access via Centre Road)

The site access via Centre Road operates at a high degree of saturation as a result of the significant through volumes on Warrigal Road and the queues that extend from the intersection of Centre Road/Warrigal Road.

The amendments to the phasing at Centre Road/Warrigal Road intersection result in a higher degree of saturation at this intersection as well as longer queues.

Significantly, the average delay in the post development scenario is not significant and indicates that the intersection will continue to operate effectively.

k.	Degree of Saturation		Average Delay (sec)		95 th Percentile queue (m)			
Approach	Existing	Post Development	Existing	Post Development	Existing	Post Development		
	AM Peak Hour							
South Warrigal Road	0.941	0.941	47.5 sec	48.5 sec	390.1m	397.7m		
East Centre Road	0.867	0.881	59.7 sec	62.2 sec	174.0m	186.2m		
North Warrigal Road	0.900	0.906	45.6 sec	46.7 sec	236.6m	236.6m		
West Centre Road	0.950	0.954	62.6 sec	64.0 sec	146.5m	150.6m		
PM Peak Hour								
South Warrigal Road	1.012	1.026	84.6 sec	92.0 sec	491.7m	520.4m		
East Centre Road	0.861	0.851	60.2 sec	59.2 sec	175.3m	177.7m		
North Warrigal Road	1.038	1.051	110.8 sec	112.5 sec	236.6sm	236.6m		
West Centre Road	1.041	1.053 (+.012)	97.4 sec	99.3 sec (+1.9)	206.6m	206.5m		

Table 6: Summary of SIDRA Outputs (Centre Road/Warrigal Road Intersection)

The above analysis shows that the intersection of Centre Road/Warrigal Road currently operates at capacity under existing conditions.

The additional traffic that is expected to be generated by the proposed development will have no material impact to the operation of the intersection. Significantly, the output change from the existing to post development scenario is so minor that it would be within the typical daily fluctuation of traffic volumes.

There is no available road space surrounding the subject site to provide additional capacity on either Warrigal Road or Centre Road. Accordingly, at times where there are significant queues, it would be expected that new traffic will generally displace existing traffic on that section of road, or there will be a shift in travel behaviour relating to the timing of traffic to spread the traffic flow over a greater period of time.

6. Bicycle Assessment

Statutory bicycle parking requirements are set out at Clause 52.34 of the Planning Scheme.

Table 7 below sets out the statutory bicycle parking requirements for the proposed on-site uses.



Use	Measure/No.	Rate	Requirement
Employee/Resident	:		
Dwelling	173 units	1 space to every 5 dwellings	35 spaces
Shop	2,788m ²	1 space to each 600m ² of leasable floor area if the leasable floor area exceeds 1,000m ²	5 spaces
Visitor/Customer			
Dwelling	173 units	1 space to every 10 dwellings	17 spaces
Shop	2,788m ²	1 space to each 500m ² of leasable floor area if the leasable floor area exceeds 1,000m ²	6 spaces
TOTAL			63 spaces

The proposed development has a statutory requirement for 63 bicycle spaces comprising 35 resident spaces, five employee spaces and 23 visitor spaces.

A total of 212 bicycle parking spaces are provided on-site comprising 174 secure spaces within the basement and 38 visitor spaces spread throughout the site at horizontal rails.

Accordingly, the bicycle parking requirements set out at Clause 52.34 of the Planning Scheme is satisfied.



7. Loading

A loading area is provided on the west side of the development site which is accessed via the east-west section of Links Avenue.

The loading area includes provision of a supermarket loading dock for a 19m semi-trailer and a secondary loading area to facilitate loading activities and waste collection for the smaller commercial uses.

Waste collection for the residential use is to be collected within the basement by a 6.4m waste wise mini (or similar sized collection vehicle).

Traffix Group has prepared swept path diagrams (attached at Appendix C) to demonstrate access to the loading bays and waste collection arrangements.

We note that minor works may be required to a small number of existing car parking spaces in order to facilitate semi-trailer movements.

The proposed loading and waste collection arrangements are appropriate and will not adversely impact traffic flow or road safety and importantly, will work well.



8. Conclusion

Having undertaken a traffic engineering assessment of the proposed mixed-use development at 1041 Centre Road (Links Shopping Centre), Oakleigh, we are of the opinion that:

- a) the proposed development has a statutory car parking requirement for 298 spaces,
- b) the provision of 238 spaces comprising 178 residential spaces and 60 commercial spaces falls short of the commercial car parking requirement and accordingly, a permit to reduce the car parking requirement is being sought as part of this application,
- c) there is sufficient justification to warrant a reduction in the statutory car parking requirement having regard to:
 - i. the residential component of the car parking requirement is met on-site through the provision of basement car parking,
 - ii. the supermarket use will be a secondary supermarket on the site and based on our experience will generate car parking at a rate of 3.5 spaces per 100m²,
 - iii. there are approximately 52 parking spaces available on nearby land (existing centre carpark), and
 - iv. the provision of 60 new spaces in addition to the existing surplus spaces will meet the commercial car parking demand,
- d) the carpark layout is provided in accordance with the requirement of the Planning Scheme and Australian Standard (where necessary) and importantly, will work well,
- e) additional traffic created by the proposed development in the AM and PM peak periods will be able to be accommodated on the already highly congested road network without any material change the operation or efficiency of the road network,
- f) the provision of bicycle parking is sufficient to meet the statutory requirement set out in Clause 52.34 of the Planning Scheme,
- g) loading and waste collection services can be suitably provided on-site without adversely impacting traffic flow or road safety, and
- h) there are no traffic engineering reasons why a planning permit for the proposed mixeduse development at 1041 Centre Road (Links Shopping Centre), Oakleigh should not be granted.





Appendix A

Proposed Development Plan

Traffix Group

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TDO	13/08/2021	Town Planning Issue: Issued for submission	YHU
TP1	04/11/2021	Town Planning RFI dated 29/9/21 response; issued for update	MGR
TP2	17/01/2022	Town Planning RFI response; Issued for application	CLY
TP3	24/02/2022	Town Planning RFI issued for application	YHU

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TP1	17/01/2022	Town Planning RFI response; Issued for application	CLY
TP2	14/02/2022	Issued for Coordination	YHU
TP3	24/02/2022	Town Planning RFI issued for application	YHU



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TP2	14/02/2022	Issued for Coordination	YHU
TP3	24/02/2022	Town Planning RFI issued for application	YHU

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GROUND FLOOR PLAN 1:200

CLY

TP3 24/02/2022 Town Planning RFI issued for application 14/02/2022 Issued for Coordination

ISSUE / revisio

P0 13/08/2021 Town Planning Issue; Issued for submission

17/01/2022 Town Planning RFI response; Issued for application





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NOTE: AREAS ON PLAN SHOWN AS NET LETTABLE

GROUND FLOOR PLAN
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Development Schedule Town Planning Revision

Proposed Mixed Use Development Town Planning Submission Revision Date 08/04/2022

		Car Parking		Retail				Residential Apartments									
Levels	GFA sqm	Area	Car Spaces	Supermarket	Retail	Retail BoH	Amenity	Outdoor Dining	Circulation	Total Apt Number	Total RPA sqm	Studio App Number	sqm	1 Bed 1 B Number	ath App	2 Bed 1 Ba Number	athApp sqm
Basement 2 (Residential Parking) Basement 1 (Residential Parking)	4280 4280	3800 3810	92 86	0	0	0	0	0	0	0	0						
Ground Floor (Retail, Supermarket, Parking) First Floor (Residential and Terrace)	6250 3615	1540 0	60 0	1913 0	875	592 0	50 0	150	724	0 24	0 2534.5	1	46	16	856	2	=
SecondFloor (Residential Inc. Balcony) Third Floor (Residential Inc. Balcony)	2138 2165	0	0	0	0	0	0	0	0	24 25	1661.5 1748	1	46 46	16 16	856 856	2	\pm
Fourth Floor (Residential Inc. Balcony) Fifth Floor (Residential Inc. Balcony)	2165 2165	0	0	0	0	0	0	0	0	25 25	1748 1748	1	46 46	16 16	856 856	2	
Sixth Floor (Residential Inc. Balcony) Seventh Floor (Residential Inc. Balcony)	2165 2165	0	0	0	0		0		0	25 25	1748 1748	1	46	16 16	856 856	2	
[Eighth Floor (Rooftop Terrace) [Totals	882 32270	0 9150	0 238	0 1913	0 875	0 592	0 50	150	0 724	0 173	0 12,936	7	0	2 112	0	14	\pm

	Studio	1 Bed	2 Bed
Apartment Number	7	112	
Apartment Average Size	46.0	53.5	
Apartment Mix (by number)	4.0%	64.7%	
Apartment Mix (by area)	2.5%	46.3%	
Apartment Parking	7	112	

Development Breakdown	
Site Area	
GFA	32,270
Total Residential Apartments	173
Total Residential GFA	16,578
Typical RPA	1,748
Residential Efficency	74%
Total Car Parking Spaces	238

Notes and Disclaime

These areas and room numbers are approximate. They relate to areas of the building at the current stage of design and are reliant upon the information available. All areas in sqm. All areas subject to survey.

Parking Totals

Car Parking]	238
Bike Parking]	212



р	2 Bed 2 BathApp		2 Bed 2 Bat	hApp CR	Balcony	Indoor Res	Outdoor Res
	Number	sqm	Number	sqm	sqm	Amenity	Amenity
					0		
					0		
					0	127	
140	3	232.5	2	180	1080	122	1840
140	3	232.5	2	180	207	59	
140	4	310	2	180	216		
140	4	310	2	180	216		
140	4	310	2	180	216		
140	4	310	2	180	216		
140	4	310	2	180	216		
0		0		0	0	316	255
980	26	2,015	14	1,260	2367		

	2 Bed		2 Bed Custom	
14		26		14
70.0		77.5		90.0
8.1%		15.0%		8.1%
7.6%		15.6%		9.7%
14		26		14

Visitor	Total Parking
35	208

i2 | Ryder



Appendix B

SIDRA Outputs



G29458R-01C

Site: 101 [Warrigal Access - Existing (Site Folder: General)]

■ Network: N101 [AM Existing (Network Folder: General)]

Warrigal Access - Existing Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehicle Movement Performance														
Mov ID	Turn	DEMA FLOV [Total veh/h	ND VS HV] %	ARRI FLO\ [Total veh/h	VAL NS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B QU [Veh. veh	ACK OF EUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
South: Warrigal Road														
2	T1	1728	5.0	1728	5.0	0.474	0.4	LOS A	2.8	20.3	0.05	0.05	0.05	59.4
3	R2	82	1.0	82	1.0	* 0.363	7.9	LOS A	0.6	4.3	0.13	0.63	0.13	23.7
Appro	bach	1811	4.8	1811	4.8	0.474	0.7	LOS A	2.8	20.3	0.05	0.07	0.05	56.9
East:	Site Ac	cess												
4	L2	59	1.0	59	1.0	0.289	2.4	LOS A	1.0	7.4	0.26	0.22	0.26	19.1
6	R2	81	1.0	81	1.0	*0.433	69.2	LOS E	2.7	18.8	1.00	0.73	1.00	18.5
Appro	bach	140	1.0	140	1.0	0.433	41.1	LOS D	2.7	18.8	0.69	0.52	0.69	18.6
North	: Warrię	gal Road												
7	L2	128	1.0	128	1.0	0.072	5.8	LOS A	0.2	1.2	0.02	0.58	0.02	33.7
8	T1	1657	5.0	1657	5.0	* 0.892	12.2	LOS B	15.7	115.0	0.17	0.23	0.26	43.0
Appro	bach	1785	4.7	1785	4.7	0.892	11.8	LOS B	15.7	115.0	0.16	0.26	0.24	41.8
All Ve	hicles	3736	4.6	3736	4.6	0.892	7.5	LOS A	15.7	115.0	0.13	0.18	0.17	46.3

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Mo	vement	Perfor	nance							
Mov ID Crossing	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE	BACK OF UE	Prop. Ef Que	fective Stop	Travel Time	Travel Dist.	Aver. Speed
	1/1			[Ped	Dist]		Rate			,
	ped/h	sec		ped	m			sec	m	m/sec
South: Warrigal F	Road									
P1 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43
East: Site Access	6									
P2 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	83.7	31.9	0.38
All Pedestrians	11	59.2	LOS E	0.0	0.0	0.95	0.95	86.2	35.2	0.41

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 103 [Centre Road Access - Existing (Site Folder: General)]

■ Network: N101 [AM Existing (Network Folder: General)]

Centre Road Access - Existing

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehi	cle Mo	vement	Perfo	ormano	ce									
Mov ID	Turn	DEMA FLO\ [Total veh/h	AND WS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% [Ql [Veh. veh	BACK OF UEUE Dist] m	Prop. Que	Effective <i>A</i> Stop Rate	ver. No. Cycles	Aver. Speed km/h
East:	Centre	Road												
5 6	T1 R2	735 99	5.0 1.0	735 99	5.0 1.0	0.209 * 0.906	0.3 85.2	LOS A LOS F	0.6 7.2	4.1 50.8	0.03 1.00	0.02 0.97	0.03 1.50	59.4 16.6
Appro	bach	834	4.5	834	4.5	0.906	10.4	LOS B	7.2	50.8	0.14	0.14	0.20	43.6
North	: Site A	ccess												
7 0	L2 R2	99 19	1.0 1.0	99 10	1.0 1.0	0.143	3.2 61.1	LOS A	1.4	10.1 4 0	0.28	0.23	0.28	38.6 5.6
Appro	bach	118	1.0	118	1.0	0.143	12.5	LOS B	1.4	10.1	0.39	0.30	0.39	31.5
West	Centre	e Road												
10 11	L2 T1	48 011	1.0 5.0	48 011	1.0	* 0.310 * 0.305	60.5 13.2	LOS E	2.7 20.1	19.2 146.8	0.89	0.73	0.89	14.0 46.6
Appro	bach	959	4.8	959	4.8	0.310	15.6	LOS B	20.1	146.8	0.78	0.69	0.78	43.9
All Ve	hicles	1911	4.4	1911	4.4	0.906	13.1	LOS B	20.1	146.8	0.48	0.42	0.50	42.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Mo	vement	Perfor	nance							
Mov D Crossing	Dem. Flow	Aver. Delav	Level of Service	AVERAGE		Prop. Et	fective Stop	Travel Time	Travel Dist	Aver. Speed
	1 10 11	Delay	0011100	[Ped	Dist]	Que	Rate	TITIC	Diot.	opeed
	ped/h	sec		ped	m			sec	m	m/sec
North: Site Acces	s									
P3 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	86.2	35.2	0.41
West: Centre Roa	ad									
P4 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43
All Pedestrians	11	59.2	LOS E	0.0	0.0	0.95	0.95	87.5	36.9	0.42

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 102 [Centre Road/ Warrigal Road Intersection (Site Folder: General)]

■ Network: N101 [AM Existing (Network Folder: General)]

Centre Road/ Warrigal Road Intersection Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehi	cle Mo	vement	Perfo	ormano	ce									
Mov ID	Turn	DEMA FLOV [Total veh/h	ND VS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B QU [Veh. veh	ACK OF IEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
South	: Warri	gal Road												
1 2 3 Appro	L2 T1 R2 pach	63 1615 200 1878	5.0 5.0 5.0 5.0	63 1615 200 1878	5.0 5.0 5.0 5.0	0.051 * 0.929 * 0.941 0.941	6.6 44.0 88.5 47.5	LOS A LOS D LOS F LOS D	0.2 53.4 15.3 53.4	1.3 390.1 111.9 390.1	0.05 0.88 1.00 0.86	0.56 0.91 1.03 0.91	0.05 1.02 1.49 1.04	53.4 25.0 15.6 24.3
East:	Centre	Road												
4 5	L2 T1	202 533	5.0 5.0	202 533	5.0 5.0	0.867 0.867	60.0 58.1	LOS E LOS E	23.8 23.8	174.0 174.0	1.00 1.00	1.10 1.02	1.15 1.16	26.6 26.3
Appro	bach	882	5.0	882	5.0	0.867	59.7	LOS E	23.8	174.0	1.00	1.00	1.13	24.4
North	: Warri	gal Road												
7 8 9	L2 T1 R2	126 1533 172	5.0 5.0 5.0	126 1533 172	5.0 5.0 5.0	0.093 0.900 0.808	11.2 45.0 76.9	LOS B LOS D LOS E	2.4 32.4 11.5	17.4 236.6 83.7	0.34 0.91 1.00	0.64 0.92 0.86	0.34 1.04 1.10	28.4 28.3 20.3
Appro	ach	1831	5.0	1831	5.0	0.900	45.6	LOS D	32.4	236.6	0.88	0.90	1.00	27.2
West:	Centre	e Road												
10 11 12 Appro	L2 T1 R2 pach	253 526 166 945	5.0 5.0 5.0 5.0	253 526 166 945	5.0 5.0 5.0 5.0	0.325 * 0.946 * 0.950 0.950	21.7 74.7 86.4 62.6	LOS C LOS E LOS F LOS E	8.7 20.1 12.2 20.1	63.8 146.5 89.0 146.5	0.61 0.95 0.99 0.87	0.73 1.03 1.02 0.95	0.61 1.34 1.47 1.17	35.5 17.8 24.7 22.1
All Ve	hicles	5536	5.0	5536	5.0	0.950	51.4	LOS D	53.4	390.1	0.89	0.93	1.06	24.8

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Peo	destrian Mov	/ement	Perform	nance							
Mo	/	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop. Ef	fective	Travel	Travel	Aver.
ID	Crossing	Flow	Delay	Service	QUE [Ped	UE Dist]	Que	Stop Rate	Time	Dist.	Speed
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Warrigal R	load									
P1	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43
Eas	t: Centre Roa	d									
P2	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43
Nor	th: Warrigal R	oad									
P3	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43
We	st: Centre Roa	ad									

Site: 101 [Warrigal Access - Post (Site Folder: General)]

Warrigal Access - Existing Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehio	cle Mo	vement	Perfo	rmanc	:e									
Mov ID	Turn	DEMA FLO\ [Total veh/h	AND NS HV] %	ARRI FLO [Total veh/h	VAL NS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B, QU [Veh. veh	ACK OF EUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
South	: Warri	gal Road												
2	T1	1728	5.0	1728	5.0	0.474	0.4	LOS A	2.8	20.2	0.05	0.05	0.05	59.4
3	R2	92	1.0	92	1.0	*0.384	8.7	LOS A	0.9	6.5	0.17	0.64	0.17	23.4
Appro	bach	1820	4.8	1820	4.8	0.474	0.8	LOS A	2.8	20.2	0.05	0.08	0.05	56.6
East:	East: Site Access													
4	L2	71	1.0	71	1.0	0.338	2.8	LOS A	1.3	9.3	0.29	0.25	0.29	18.9
6	R2	120	1.0	120	1.0	*0.641	70.8	LOS E	4.0	28.5	1.00	0.84	1.10	18.3
Appro	bach	191	1.0	191	1.0	0.641	45.7	LOS D	4.0	28.5	0.74	0.62	0.80	18.4
North	: Warrię	gal Road												
7	L2	144	1.0	144	1.0	0.082	5.8	LOS A	0.2	1.4	0.02	0.58	0.02	33.7
8	T1	1657	5.0	1657	5.0	* 0.901	14.4	LOS B	17.6	128.2	0.19	0.25	0.28	41.0
Appro	bach	1801	4.7	1801	4.7	0.901	13.7	LOS B	17.6	128.2	0.17	0.28	0.26	40.0
All Ve	hicles	3812	4.6	3812	4.6	0.901	9.1	LOS A	17.6	128.2	0.15	0.20	0.19	44.2

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Mo	vement	Perform	nance							
Mov Crossing	Dem.	Aver.	Level of	AVERAGE I	BACK OF	Prop. Ef	fective	Travel	Travel	Aver.
	FIOW	Delay	Service	[Ped	Dist]	Que	Rate	nne	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
South: Warrigal F	Road									
P1 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43
East: Site Access	;									
P2 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	83.7	31.9	0.38
All Pedestrians	11	59.2	LOS E	0.0	0.0	0.95	0.95	86.2	35.2	0.41

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 103 [Centre Road Access - Post (Site Folder: General)]

Centre Road Access - Existing Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehi	cle Mo	vement	Perfo	rmano	ce									
Mov ID	Turn	DEMA FLOV [Total veh/h	AND NS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B QU [Veh. veh	ACK OF EUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
East:	Centre	Road												
5	T1	735	5.0	735	5.0	0.211	0.3	LOS A	0.6	4.2	0.03	0.02	0.03	59.4
6	R2	114	1.0	114	1.0	*0.911	85.2	LOS F	8.3	58.6	1.00	0.98	1.49	16.6
Appro	bach	848	4.5	848	4.5	0.911	11.7	LOS B	8.3	58.6	0.16	0.15	0.22	42.2
North	: Site A	ccess												
7	L2	122	1.0	122	1.0	0.174	3.6	LOS A	1.9	13.5	0.30	0.25	0.30	38.4
9	R2	26	1.0	26	1.0	0.077	60.1	LOS E	0.8	5.5	0.95	0.66	0.95	5.7
Appro	bach	148	1.0	148	1.0	0.174	13.6	LOS B	1.9	13.5	0.42	0.32	0.42	30.8
West	Centre	Road												
10	L2	53	1.0	53	1.0	* 0.307	58.6	LOS E	2.9	20.4	0.87	0.73	0.87	14.3
11	T1	911	5.0	911	5.0	*0.312	14.5	LOS B	20.7	151.4	0.79	0.70	0.79	45.6
Appro	bach	963	4.8	963	4.8	0.312	16.9	LOS B	20.7	151.4	0.79	0.70	0.79	43.0
All Ve	hicles	1960	4.4	1960	4.4	0.911	14.4	LOS B	20.7	151.4	0.49	0.44	0.52	41.6

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Pedestrian Mov	vement	Perform	nance							
Mov Crossing	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop. Ef	fective	Travel	Travel	Aver.
ID crocoing	FIOW	Delay	Service	[Ped	Dist]	Que	Rate	Time	Dist.	Speed
	ped/h	sec		ped	m			sec	m	m/sec
North: Site Acces	s									
P3 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	86.2	35.2	0.41
West: Centre Roa	ad									
P4 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43
All Pedestrians	11	59.2	LOS E	0.0	0.0	0.95	0.95	87.5	36.9	0.42

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 102 [Centre Road/ Warrigal Road Intersection - Post (Site Folder: General)]

Centre Road/ Warrigal Road Intersection Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehi	cle Mo	vement	Perfo	ormano	ce									
Mov	Turn	DEMA	ND	ARRI	VAL	Deg.	Aver.	Level of	95% B.	ACK OF	Prop.	EffectiveA	ver. No.	Aver.
ID		FLOV	VS	FLO'	WS	Satn	Delay	Service	QU	EUE Diat 1	Que	Stop	Cycles	Speed
		veh/h	пvј %	veh/h	пvј %	v/c	sec		veh	m Dist j		Nale		km/h
South	n: Warri	gal Road												
1	L2	63	5.0	63	5.0	0.051	6.6	LOS A	0.2	1.3	0.05	0.56	0.05	53.4
2	T1	1623	5.0	1623	5.0	* 0.933	45.2	LOS D	54.5	397.7	0.88	0.92	1.04	24.6
3	R2	200	5.0	200	5.0	*0.941	88.5	LOS F	15.3	111.9	1.00	1.03	1.49	15.6
Appro	bach	1886	5.0	1886	5.0	0.941	48.5	LOS D	54.5	397.7	0.87	0.92	1.05	24.0
East:	Centre	Road												
4	L2	202	5.0	202	5.0	0.881	64.1	LOS E	25.5	186.2	1.00	1.13	1.20	25.6
5	T1	541	5.0	541	5.0	0.881	60.7	LOS E	25.5	186.2	1.00	1.04	1.20	25.6
6	R2	147	5.0	147	5.0	0.607	65.3	LOS E	9.0	65.9	0.99	0.80	0.99	11.8
Appro	bach	891	5.0	891	5.0	0.881	62.2	LOS E	25.5	186.2	1.00	1.02	1.16	23.8
North	: Warrię	gal Road												
7	L2	126	5.0	126	5.0	0.094	11.2	LOS B	2.4	17.5	0.34	0.64	0.34	28.4
8	T1	1544	5.0	1544	5.0	0.906	46.3	LOS D	32.4	236.6	0.92	0.94	1.05	27.8
9	R2	172	5.0	172	5.0	0.808	76.9	LOS E	11.5	83.7	1.00	0.86	1.10	20.3
Appro	bach	1842	5.0	1842	5.0	0.906	46.7	LOS D	32.4	236.6	0.88	0.91	1.01	26.9
West	: Centre	e Road												
10	L2	253	5.0	253	5.0	0.326	21.7	LOS C	8.8	64.0	0.61	0.73	0.61	35.5
11	T1	531	5.0	531	5.0	* 0.954	77.1	LOS E	20.6	150.6	0.95	1.05	1.37	17.4
12	R2	166	5.0	166	5.0	*0.952	86.8	LOS F	12.2	89.2	0.99	1.02	1.47	24.6
Appro	bach	949	5.0	949	5.0	0.954	64.0	LOS E	20.6	150.6	0.87	0.96	1.18	21.8
All Ve	hicles	5568	5.0	5568	5.0	0.954	52.8	LOS D	54.5	397.7	0.89	0.94	1.08	24.4

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Peo	destrian Mov	vement	Perform	nance							
Mo∖ ID	/ Crossing	Dem. Flow	Aver. Delav	Level of Service	AVERAGE E	BACK OF JE	Prop. Ef Que	fective Stop	Travel Time	Travel Dist.	Aver. Speed
		1/1			[Ped	Dist]		Rate			'
		ped/n	sec		ped	m			sec	m	m/sec
Sou	th: Warrigal F	Road									
P1	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43
Eas	t: Centre Roa	d									
P2	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43
Nor	th: Warrigal R	oad									
P3	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43

Site: 101 [Warrigal Access - PM Existing (Site Folder: General)]

■ Network: N101 [PM Existing (Network Folder: General)]

Warrigal Access - PM Existing

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehi	cle Mo	vement	Perfo	ormand	ce									
Mov ID	Turn	DEMA FLOV [Total veh/h	ND NS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% [Ql [Veh. veh	BACK OF JEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
South	: Warri	gal Road												
2 3	T1 R2	1765 117	5.0 1.0	1749 116	5.0 1.0	0.480	0.4	LOS A LOS A	3.3 1.0	24.2 6.8	0.05	0.05	0.05	59.3 23.7
Appro	bach	1882	4.8	1865 1	4.8	0.480	0.9	LOSA	3.3	24.2	0.06	0.09	0.06	56.0
East:	Site Ac	cess												
4	L2	98	1.0	98	1.0	0.442	2.7	LOS A	4.8	33.6	0.31	0.27	0.31	18.9
6	R2	182	1.0	182	1.0	* 0.973	93.2	LOS F	7.2	51.1	1.00	1.46	1.73	16.0
Appro	bach	280	1.0	280	1.0	0.973	61.6	LOS E	7.2	51.1	0.76	1.04	1.23	16.4
North	: Warrię	gal Road												
7	L2	172	1.0	172	1.0	0.098	5.8	LOS A	0.2	1.6	0.02	0.59	0.02	33.7
8	T1	1604	5.0	1604	5.0	* 0.881	10.2	LOS B	39.0	284.5	0.16	0.21	0.23	45.1
Appro	bach	1776	4.6	1776	4.6	0.881	9.8	LOS A	39.0	284.5	0.15	0.24	0.21	43.1
All Ve	hicles	3938	4.4	<mark>3921</mark> N 1	4.4	0.973	9.3	LOS A	39.0	284.5	0.15	0.23	0.21	43.2

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Pedestrian Movement Performance													
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop. Ef	ffective	Travel	Travel	Aver.			
ID Crossing	Flow	Delay	Service	QUEUE [Ped Dist]		Que	Stop Rate	Time	Dist.	Speed			
	ped/h	sec		ped	m			sec	m	m/sec			
South: Warrigal F	Road												
P1 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43			
East: Site Access	6												
P2 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	83.7	31.9	0.38			
All Pedestrians	11	59.2	LOS E	0.0	0.0	0.95	0.95	86.2	35.2	0.41			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 103 [Centre Road Access - PM Existing (Site Folder: General)]

■ Network: N101 [PM Existing (Network Folder: General)]

Centre Road Access - PM Existing Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehi	cle Mo	vement	Perfo	ormand	ce									
Mov ID	Turn	DEMA FLO\ [Total veh/h	AND NS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Q [Veh. veh	BACK OF UEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
East:	Centre	Road												
5	T1	632	5.0	632	5.0	0.186	0.3	LOS A	0.5	3.5	0.03	0.02	0.03	59.4
6	R2	162	1.0	162	1.0	*0.358	51.8	LOS D	8.1	57.1	0.83	0.78	0.83	22.0
Appro	bach	794	4.2	794	4.2	0.358	10.9	LOS B	8.1	57.1	0.19	0.18	0.19	42.3
North: Site Access														
7	L2	196	1.0	196	1.0	0.216	3.6	LOS A	3.2	22.8	0.32	0.26	0.32	38.4
9	R2	59	1.0	59	1.0	0.135	57.4	LOS E	1.7	12.1	0.94	0.68	0.94	5.9
Appro	bach	255	1.0	255	1.0	0.216	16.1	LOS B	3.2	22.8	0.46	0.36	0.46	28.9
West:	Centre	e Road												
10	L2	78	1.0	76	1.0	*0.348	69.6	LOS E	4.8	34.0	1.00	0.77	1.00	12.8
11	T1	803	5.0	783	5.0	*0.362	26.2	LOS C	20.5	150.0	0.86	0.75	0.86	38.2
Appro	bach	881	4.6	<mark>859</mark> ^{N1}	4.6	0.362	30.0	LOS C	20.5	150.0	0.87	0.75	0.87	35.0
All Ve	hicles	1929	4.0	1907 ^N	4.0	0.362	20.2	LOS C	20.5	150.0	0.53	0.46	0.53	36.4

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Pedestrian Movement Performance														
Mov	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop. Ef	fective	Travel	Travel	Aver.				
ID Crossing	FIOW	Delay	Service	QUEUE [Ped Dist]		Que	Stop Rate	IIme	Dist.	Speed				
	ped/h	sec		ped	m			sec	m	m/sec				
North: Site Acces	s													
P3 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	86.2	35.2	0.41				
West: Centre Roa	ad													
P4 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43				
All	11	59.2	LOS E	0.0	0.0	0.95	0.95	87.5	36.9	0.42				
Pedestrians														

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 102 [Centre Road/ Warrigal Road Intersection - PM Existing (Site Folder: General)]

■ Network: N101 [PM Existing (Network Folder: General)]

Centre Road/ Warrigal Road Intersection Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehi	Vehicle Movement Performance Mov Turn DEMAND ARRIVAL Deg. Aver. Level of 95% BACK OF Prop. EffectiveAver. No. Aver.													
Mov ID	Turn	DEMA FLOV	ND VS	ARRI FLO	VAL WS	Deg. Satn	Aver. Delay	Level of Service	95% E Ql	BACK OF	Prop. Que	EffectiveA Stop	ver. No. Cycles	Aver. Speed
		[Total veh/h	HV] %	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate		km/h
South	: Warri	gal Road												
1	L2	76	5.0	76	5.0	0.059	7.6	LOS A	0.4	3.0	0.10	0.57	0.10	52.6
2	T1	1491	5.0	1491	5.0	1.012	90.1	LOS F	67.4	491.7	1.00	1.25	1.44	15.4
3	R2	155	5.0	155	5.0	0.728	69.3	LOS E	9.9	71.9	1.00	0.84	1.07	18.6
Appro	bach	1721	5.0	1721	5.0	1.012	84.6	LOS F	67.4	491.7	0.96	1.18	1.35	16.7
East:	Centre	Road												
4	L2	189	5.0	189	5.0	0.861	63.5	LOS E	24.0	175.3	1.00	1.13	1.12	25.7
5	T1	551	5.0	551	5.0	0.861	60.0	LOS E	24.0	175.3	1.00	1.05	1.17	25.8
6	R2	158	5.0	158	5.0	0.434	56.7	LOS E	8.9	64.7	0.92	0.79	0.92	13.2
Appro	bach	898	5.0	898	5.0	0.861	60.2	LOS E	24.0	175.3	0.99	1.02	1.12	24.2
North	: Warri	gal Road												
7	L2	114	5.0	114	5.0	0.087	12.9	LOS B	2.4	17.4	0.37	0.64	0.37	26.4
8	T1	1461	5.0	1461	5.0	* 1.038	116.3	LOS F	32.4	236.6	1.00	1.39	1.63	15.2
9	R2	220	5.0	220	5.0	* 1.036	124.9	LOS F	19.8	144.3	1.00	1.10	1.62	14.2
Appro	bach	1795	5.0	1795	5.0	1.038	110.8	LOS F	32.4	236.6	0.96	1.31	1.55	15.2
West	Centre	e Road												
10	L2	278	5.0	278	5.0	0.391	26.4	LOS C	11.3	82.2	0.69	0.76	0.69	32.6
11	T1	571	5.0	571	5.0	* 1.041	123.9	LOS F	28.3	206.6	1.00	1.29	1.74	12.0
12	R2	244	5.0	244	5.0	* 1.016	116.0	LOS F	21.9	159.8	1.00	1.13	1.66	20.5
Appro	bach	1093	5.0	1093	5.0	1.041	97.4	LOS F	28.3	206.6	0.92	1.12	1.45	16.7
All Ve	hicles	5506	5.0	5506	5.0	1.041	91.7	LOS F	67.4	491.7	0.96	1.18	1.40	17.1

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Peo	Pedestrian Movement Performance														
Mo		Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop. Et	ffective	Travel	Travel	Aver.				
ID	Crossing	Flow	Delay	Service	QUEUE [Ped Dist]		Que	Stop Rate	Time	Dist.	Speed				
		ped/h	sec		ped	m			sec	m	m/sec				
South: Warrigal Road															
P1	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43				
Eas	t: Centre Road	b													
P2	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43				
Nor	th: Warrigal Ro	oad													
P3	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43				
We	st: Centre Roa	d													

Site: 101 [Warrigal Access - PM Post (Site Folder: General)]

Warrigal Access - PM Existing Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehio	cle Mo	vement	Perfo	ormano	e:									
Mov ID	Turn	DEMA FLO\ [Total veh/h	AND NS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B QU [Veh. veh	ACK OF EUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
South	: Warri	gal Road	l											
2 3	T1 R2	1765 140	5.0 1.0	1730 137	5.0 1.0	0.483 * 0.474	0.5 8.8	LOS A LOS A	3.7 1.5	27.0 10.8	0.06 0.18	0.05 0.65	0.06 0.18	59.2 23.4
Appro	bach	1905	4.7	1867 ^N 1	4.7	0.483	1.1	LOS A	3.7	27.0	0.07	0.10	0.07	55.2
East:	Site Ac	cess												
4 6	L2 R2	119 233	1.0 1.0	119 233	1.0 1.0	0.482 * 0.932	3.0 82.6	LOS A	5.9 8.7	41.5 61.4	0.34 1.00	0.30 1.33	0.34 1.55	18.9 17.0
Appro	bach	352	1.0	352	1.0	0.932	55.7	LOS E	8.7	61.4	0.78	0.98	1.14	17.3
North	: Warrię	gal Road												
7	L2	224	1.0	224	1.0	0.131	5.9	LOS A	0.3	2.2	0.02	0.59	0.02	33.7
8	T1	1556	5.0	1556	5.0	* 0.890	12.4	LOS B	38.6	281.8	0.17	0.23	0.26	42.8
Appro	bach	1780	4.5	1780	4.5	0.890	11.6	LOS B	38.6	281.8	0.15	0.28	0.23	40.9
All Ve	hicles	4037	4.3	<mark>3999</mark> N	4.3	0.932	10.6	LOS B	38.6	281.8	0.17	0.25	0.23	41.3

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Dor	Pedestrian Movement Performance													
Feu		vement	Fellon	liance										
Mov	/	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop. E	ffective	Travel	Travel	Aver.			
ID	Crossing	Flow	Delay	Service	QUEUE		Que	Stop	Time	Dist.	Speed			
					[Ped Dist]			Rate						
		ped/h	sec		ped	m			sec	m	m/sec			
South: Warrigal Road														
P1	Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43			
Eas	t: Site Access	;												
P2	Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	83.7	31.9	0.38			
All Ped	lestrians	11	59.2	LOS E	0.0	0.0	0.95	0.95	86.2	35.2	0.41			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 103 [Centre Road Access - PM Post (Site Folder: General)]

■ Network: N101 [PM Post Development (Network Folder: General)]

Centre Road Access - PM Existing Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehi	cle Mo	vement	Perfo	ormano	e:									
Mov ID	Turn	DEMA FLO\ [Total veh/h	AND WS HV] %	ARRI FLO [Total veh/h	VAL WS HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% E QL [Veh. veh	BACK OF JEUE Dist] m	Prop. Que	EffectiveA Stop Rate	ver. No. Cycles	Aver. Speed km/h
East:	Centre	Road												
5 6	T1 R2	632 203	5.0 1.0	632 203	5.0 1.0	0.213 * 0.895	0.4 79.7	LOS A LOS E	0.5 14.7	3.6 103.5	0.03 1.00	0.02 0.97	0.03 1.35	59.2 17.3
Appro	bach	835	4.0	835	4.0	0.895	19.7	LOS B	14.7	103.5	0.26	0.25	0.35	35.3
North	North: Site Access													
7 9	L2 R2	231 73	1.0 1.0	231 73	1.0 1.0	0.275 0 142	4.8 53.6	LOS A	4.5 2.0	31.9 14 5	0.37 0.91	0.32	0.37 0.91	37.6 6.2
Appro	bach	303	1.0	303	1.0	0.275	16.5	LOS B	4.5	31.9	0.50	0.40	0.50	28.6
West	Centre	e Road												
10	L2	92	1.0	90	1.0	* 0.352	61.2	LOS E	5.0	35.2	0.90	0.76	0.90	13.9
11	T1	803	5.0	788	5.0	* 0.359	23.4	LOS C	20.9	152.5	0.85	0.75	0.85	39.7
Appro	bach	895	4.6	877 ^{N1}	4.6	0.359	27.3	LOS C	20.9	152.5	0.86	0.75	0.86	36.2
All Ve	hicles	2033	3.8	2015 ^N	3.9	0.895	22.5	LOS C	20.9	152.5	0.56	0.49	0.59	34.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Pedestrian Movement Performance														
Mov Crossing	Dem.	Aver.	Level of	AVERAGE		Prop. Ef	fective	Travel	Travel	Aver.				
	FIOW	Delay	Service	QUEUE [Ped Dist]		Que	Rate	TITLE	DISI.	Speed				
	ped/h	sec		ped	m			sec	m	m/sec				
North: Site Acces	s													
P3 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	86.2	35.2	0.41				
West: Centre Roa	ad													
P4 Full	5	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43				
All Pedestrians	11	59.2	LOS E	0.0	0.0	0.95	0.95	87.5	36.9	0.42				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 102 [Centre Road/ Warrigal Road Intersection - PM Post (Site Folder: General)]

■ Network: N101 [PM Post Development (Network Folder: General)]

Centre Road/ Warrigal Road Intersection Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 130 seconds (Network User-Given Cycle Time)

Vehicle Movement Performance Mov Turn DEMAND ARRIVAL Deg. Aver. Level of 95% BACK OF Prop. EffectiveAver. No. Aver.														
Mov	Turn	DEMA	ND	ARRI	VAL	Deg.	Aver.	Level of	95% E	BACK OF	Prop.	EffectiveA	ver. No.	Aver.
ID		FLOV	VS 山\/1	FLO'	WS ЦV 1	Satn	Delay	Service	Ql [\/eh	JEUE Diet 1	Que	Stop Rate	Cycles	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Trate		km/h
South	n: Warri	gal Road												
1	L2	76	5.0	76	5.0	0.059	7.9	LOS A	0.5	3.4	0.11	0.57	0.11	52.4
2	T1	1514	5.0	1514	5.0	1.026	98.5	LOS F	71.3	520.4	1.00	1.30	1.50	14.4
3	R2	155	5.0	155	5.0	0.728	69.6	LOS E	10.0	73.2	1.00	0.85	1.10	18.5
Appro	bach	1744	5.0	1744	5.0	1.026	92.0	LOS F	71.3	520.4	0.96	1.23	1.41	15.6
East:	Centre	Road												
4	L2	189	5.0	189	5.0	0.851	63.7	LOS E	24.3	177.7	1.00	1.14	1.14	25.7
5	T1	564	5.0	564	5.0	0.851	58.0	LOS E	24.3	177.7	0.99	1.02	1.13	26.3
6	R2	158	5.0	158	5.0	0.452	57.7	LOS E	8.9	65.3	0.93	0.80	0.93	13.0
Appro	bach	912	5.0	912	5.0	0.851	59.2	LOS E	24.3	177.7	0.98	1.01	1.10	24.5
North	: Warrię	gal Road												
7	L2	114	5.0	114	5.0	0.088	13.5	LOS B	2.3	16.7	0.36	0.63	0.36	25.7
8	T1	1482	5.0	1482	5.0	* 1.051	118.3	LOS F	32.4	236.6	1.00	1.39	1.62	15.0
9	R2	220	5.0	220	5.0	* 1.036	124.9	LOS F	19.8	144.3	1.00	1.10	1.62	14.2
Appro	bach	1816	5.0	1816	5.0	1.051	112.5	LOS F	32.4	236.6	0.96	1.31	1.54	15.0
West	Centre	e Road												
10	L2	278	5.0	278	5.0	0.387	26.3	LOS C	11.2	81.8	0.69	0.76	0.69	32.6
11	T1	584	5.0	584	5.0	* 1.031	117.5	LOS F	28.3	206.5	1.00	1.27	1.69	12.5
12	R2	244	5.0	244	5.0	* 1.053	138.7	LOS F	24.2	176.7	1.00	1.19	1.81	18.0
Appro	bach	1106	5.0	1106	5.0	1.053	99.3	LOS F	28.3	206.5	0.92	1.12	1.47	16.4
All Ve	hicles	5578	5.0	5578	5.0	1.053	94.8	LOS F	71.3	520.4	0.96	1.20	1.41	16.6

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

Peo	destrian Mov	vement	Perform	nance							
Mo\ ID	/ Crossing	Dem. Flow	Aver. Delav	Level of Service	vel of AVERAGE BACK OF F rvice QUEUE		Prop. Ef Que	fective Stop	Travel Time	Travel Dist.	Aver. Speed
					[Ped Dist]			Rate			
		ped/h	sec		ped	m			sec	m	m/sec
Sou	th: Warrigal F	Road									
P1	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43
Eas	t: Centre Roa	d									
P2	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43
Nor	th: Warrigal R	oad									
P3	Full	11	59.2	LOS E	0.0	0.0	0.95	0.95	88.8	38.5	0.43



Appendix C

Swept Path Diagrams

Traffix Group

G29458R-01C



SCALE: 1.250 (A3) 29458-01.dwg TEL: (03) 9822-2888

VEHICLE PROFILE

LOADING - INGRESS









LOADING - INGRESS





LOADING - EGRESS



REVDATEA29/07/2021B18/03/2022

NOTES

DESIGNED BY CHECKED BY J. PLACE D. MILDER J. PLACE

LINKS SHOPPING CENTRE, OAKLEIGH SOUTH PROPOSED MIXED USE DEVELOPMENT

GENERAL NOTES:

FILE NAME: 29458-02 SHEET NO.: 01





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